## **Shear Design Example 2**

The beam shown in the Figure below carries a total factored load (including self-weight) of 50 kN/m. The beam is reinforced through its length with  $\varphi$ 8 vertical stirrups at spacing of 200 mm. The beam width is 300 mm and effective depth, d is 540 mm. Use f'<sub>c</sub> = 25 MPa, and f<sub>V</sub> = 420 MPa, f<sub>ys</sub>= 300 MPa.



Assume the Span 6m is the clear span (between support face)

- a. Calculate V<sub>c</sub>? <u>Ans:</u>  $V_c = \frac{\sqrt{f_c}}{6} b_w d = 135 \ kN$
- **b.** Calculate V<sub>s?</sub> <u>Ans:</u> Av= 100.53 mm<sup>2</sup> ,  $V_s = \frac{A_v f_y d}{s}$  = 81.43 kN
- c. Calculate  $V_{n?}$  Ans:  $V_n = V_c + V_s = 216.43$  km

d. Calculate the minimum area of shear reinforcement required according to SBC 304 provisions?

$$A_{v,\min} = \max\{\frac{b_{W}s}{3f_{y}}, \frac{\sqrt{f_{C}b_{W}s}}{16f_{y}}\}\$$
  
 $A_{v,\min} = 66.7 \ mm^{2}$   $A_{v} = 100.53 \ mm^{2} > A_{v}, \ min \ O.K$ 

e. Prove that the shear reinforcement provided is sufficient according to SBC 304 provisions?

 $V_u$  at face of the support = 50\*6/2 = 150 kN

 $V_{u@d}$  at d from face of the support = 150 - 60\*0.54 = 123 kN

 $\phi V_n = 162.3 > V_u \otimes d$  Ok , yes the shear reinforcement provided is sufficient